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Authorised and notified according to Article 29 of the Regulation (EU) No 305/2011 of the European Parliament and of the Council of 9 March 2011

MEMBER OF EOTA

European Technical Assessment ETA-11/0496 of 2018/11/06

I General Part

Technical Assessment Body issuing the ETA and designated according to Article 29 of the Regulation (EU) No 305/2011: ETA-Danmark A/S

Trade name of the construction product:	Rotho Blaas TITAN Angle Brackets
Product family to which the above construction product belongs:	Three-dimensional nailing plate (Angle Bracket for timber-to-timber or timber-to-concrete or steel connections)
Manufacturer:	Rotho Blaas s.r.l Via dell'Adige 2/1 IT-39040 Cortaccia (BZ) Tel. + 39 0471 818400 Fax + 39 0471 818484 Internet www.rothoblaas.com
Manufacturing plant:	Rotho Blaas s.r.l Manufacturing Plants: T1, T2, T3
This European Technical Assessment contains:	27 pages including 2 annexes which form an integral part of the document
This European Technical Assessment is issued in accordance with Regulation (EU) No 305/2011, on the basis of:	Guideline for European Technical Approval (ETAG) No. 015 Three Dimensional Nailing Plates, April 2013, used as European Assessment Document (EAD).
This version replaces:	The previous ETA with the same number and issued on 2014-10-31

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II SPECIFIC PART OF THE EUROPEAN TECHNICAL ASSESSMENT

1 Technical description of product and intended use

Technical description of the product
Rotho Blaas srl. angle brackets are one-piece non-welded, face-fixed angle brackets to be used in timber to timber or in timber to concrete or timber to steel connections. They are connected to construction members made of timber or wood-based products with threaded (ring shank) nails according to EN 14592 or ETA or screws type LBS according to ETA-11/0030 or screws type HBS+ according to ETA-11/0030 or screws type VGS according to ETA-11/0030 or bolts according to EN 14592 and to concrete or steel members with bolts or metal anchors.

The angle brackets with a steel plate thickness of up to 4 mm are made from the following materials:

- steel S355 / Z 275 or FeZn12c according to EN 10025:2005 with $R_e \geq 355$ N/mm², $R_m \leq 630$ N/mm² and $A_{80} \geq 22\%$
- steel S235 / Z 275 or FeZn12c according to EN 10025:2005 with $R_e \geq 235$ N/mm², $R_m \leq 510$ N/mm² and $A_{80} \geq 26\%$
- steel S275 / Z 275 or FeZn12c according to EN 10025:2005 with $R_e \geq 275$ N/mm², $R_m \leq 560$ N/mm² and $A_{80} \geq 23\%$
- steel DX51D / Z275 according to EN 10346:2015 with $R_e \geq 220$ N/mm², $R_m \leq 500$ N/mm² and $A_{80} \geq 22\%$
- steel S250GD / Z275 according to EN 10346:2015 with $R_e \geq 250$ N/mm², $R_m \leq 470$ N/mm² and $A_{80} \geq 19\%$
- stainless steel with $R_e \geq 355$ N/mm², $R_m \leq 630$ N/mm² and $A_{80} \geq 22\%$

Dimensions, hole positions and typical installations are shown in Annex B. Rotho Blaas srl. angle brackets are made from steel with tolerances according to EN 10143.

2 Specification of the intended use in accordance with the applicable EAD

The angle brackets are intended for use in making connections in load bearing timber structures, as a connection between a beam and a purlin, or as a connection between wall and floor elements or as wall-to-wall connection and on concrete/steel elements, where requirements for mechanical resistance and stability and safety in use in the sense of the Basic Works Requirements 1 and 4 of Regulation (EU) 305/2011 shall be fulfilled.

The connection may be with a single angle bracket or with an angle bracket on each side of the fastened timber member (see Annex B).

The static and kinematical behaviour of the timber members or the supports shall be as described in Annex A and B.

The wood members may be of solid timber, glued laminated timber and similar glued members, or wood-based structural members with a characteristic density from 290 kg/m³ to 420 kg/m³. The wood members may be of Laminated Veneer Lumber (LVL) with a characteristic density up to 500 kg/m³ with nails/screws
in the wide face of the LVL component. This requirement to the material of the wood members can be fulfilled by using the following materials:

- Structural solid timber according to EN 14081,
- Glulam according to EN 14080,
- Glued solid timber according to EN14080,
- LVL according to EN 14374 or ETA,
- Parallam PSL,
- Intrallam LSL,
- Cross laminated timber according to ETA,
- Plywood according to EN 636 or ETA.

Annex B states the load-carrying capacities of the angle bracket connections for a characteristic density of 350 kg/m³. For timber or wood based material with a lower or higher characteristic density than 350 kg/m³ the load-carrying capacities shall be converted by the factor k_{dens}:

In load case F_1:

$$k_{dens} = \left(\frac{\rho_k}{350}\right)^{0,5} \text{ for } 290 \text{ kg/m}^3 \leq \rho_k \leq 350 \text{ kg/m}^3$$

$$k_{dens} = \left(\frac{\rho_k}{350}\right)^{0,5} \text{ for } 350 \text{ kg/m}^3 \leq \rho_k \leq 420 \text{ kg/m}^3$$

$$k_{dens} = \left(\frac{\rho_k}{350}\right)^{0,5} \text{ for LVL with } \rho_k \leq 500 \text{ kg/m}^3$$

In load case $F_{2/3}$ and $F_{4/5}$:

$$k_{dens} = \left(\frac{\rho_k}{350}\right)^{2} \text{ for } 290 \text{ kg/m}^3 \leq \rho_k \leq 350 \text{ kg/m}^3$$

$$k_{dens} = \left(\frac{\rho_k}{350}\right)^{0,5} \text{ for } 350 \text{ kg/m}^3 \leq \rho_k \leq 420 \text{ kg/m}^3$$

$$k_{dens} = \left(\frac{\rho_k}{350}\right)^{0,5} \text{ for LVL with } \rho_k \leq 500 \text{ kg/m}^3$$

where ρ_k is the characteristic density of the timber in kg/m^3.

If a wood-based panel interlayer with a thickness of not more than 26 mm is placed between the connector plate and the timber member, the lateral load-carrying capacity of the nail or screw, respectively, has to take into account the effect of the interlayer.

The design of the connections shall be in accordance with Eurocode 5 or a similar national Timber Code. The wood members shall have a thickness which is larger than the penetration depth of the nails into the members.
The angle brackets are primarily for use in timber structures subject to the dry, internal conditions defined by service classes 1 and 2 of Eurocode 5 and for connections subject to static or quasi-static loading.

The angle brackets can also be used in outdoor timber structures, service class 3, when a corrosion protection in accordance with Eurocode 5 is applied, or when stainless steel with similar or better characteristic yield strength and ultimate strength is employed.

The angle brackets may also be used for connections between a timber member and a member of concrete or steel (TITAN TCN, TCS and TCF).

The scope of the angle brackets regarding resistance to corrosion shall be defined according to national provisions that apply at the installation site considering environmental conditions and in conjunction with the admissible service conditions according to EN 1995-1-1 and the admissible corrosivity category as described and defined in EN ISO 12944-2

The provisions made in this European Technical Assessment are based on an assumed intended working life of the angle brackets of 50 years.

The indications given on the working life cannot be interpreted as a guarantee given by the producer or Assessment Body, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product and references to the methods used for its assessment

Characteristic	Assessment of characteristic
3.1 Mechanical resistance and stability*) (BWR1)	
Characteristic load-carrying capacity	See Annex B
Stiffness	No performance assessed
Ductility in cyclic testing	No performance assessed
3.2 Safety in case of fire (BWR2)	
Reaction to fire	The angle brackets are made from steel classified as Euroclass A1 in accordance with EN 13501-1 and Commission Delegated Regulation 2016/364
3.3 Hygiene, health and the environment (BWR3)	
Influence on air quality	No dangerous materials
3.7 Sustainable use of natural resources (BWR7)	No performance assessed
3.8 General aspects related to the performance of the product	The angle brackets have been assessed as having satisfactory durability and serviceability when used in timber structures using the timber species described in Eurocode 5 and subject to the conditions defined by service class 1 and 2
Identification	See Annex A

*) See additional information in section 3.9 – 3.12.

In addition to the specific clauses relating to dangerous substances contained in this European technical Assessment, there may be other requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Regulation, these requirements need also to be complied with, when and where they apply.

3.9 Methods of verification

Safety principles and partial factors

The characteristic load-carrying capacities are based on the characteristic values of the nail or screw connections and the steel plates. To obtain design values the capacities have to be divided by different partial factors for the material properties, in case of timber failure in addition multiplied with the coefficient k_{mod}.

According to EN 1990 (Eurocode – Basis of design) paragraph 6.3.5 the design value of load-carrying capacity may be determined by reducing the characteristic values of the load-carrying capacity with different partial factors.

Thus, the characteristic values of the load–carrying capacity are determined also for timber failure $F_{Rk,H}$ (obtaining the embedment strength of fasteners subjected to shear or the withdrawal capacity of the most loaded fastener, respectively) as well as for steel plate failure $F_{Rk,S}$. The design value of the load–carrying capacity is the smaller value of both load–carrying capacities.

$$F_{Rd} = \min\left\{\frac{k_{mod} \cdot F_{Rk,H}}{\gamma_{M,H}}; \frac{F_{Rk,S}}{\gamma_{M,S}}\right\}$$

Therefore, for timber failure the load duration class and the service class are included. The different partial factors γ_M for steel or timber, respectively, are also correctly taken into account.

3.10 Mechanical resistance and stability

See annex B for the characteristic load-carrying capacity in the different directions F_1, F_2, F_3, F_4 and F_5

The characteristic capacities of the angle brackets are determined by calculation assisted by testing and testing as described in the EOTA Guideline 015 clause 2.4.1. They should be used for designs in accordance with Eurocode 5 or a similar national Timber Code.

No performance has been determined in relation to ductility of a joint under cyclic testing. The contribution to the performance of structures in seismic zones, therefore, has not been assessed.

Other connector nails or screws according to EN 14592 or ETA with the same or better performance than the fasteners given in table A.4 may be used.

3.11 Aspects related to the performance of the product

3.11.1 Corrosion protection in service class 1, 2 and 3.

In accordance with ETAG 015 the angle brackets are produced from:

- steel S355 / Z 275 or FeZn12c treated according to EN 10025:2005 with $R_e \geq 355$ N/mm², $R_m \leq 630$ N/mm² and $A_{80} \geq 22\%$
- steel S235 / Z 275 or FeZn12c treated according to EN 10025:2005 with $R_e \geq 235$ N/mm², $R_m \leq 510$ N/mm² and $A_{80} \geq 26\%$
- steel S275 / Z 275 or FeZn12c treated according to EN 10025 with $R_e \geq 275$ N/mm², $R_m \leq 560$ N/mm² and $A_{80} \geq 23\%$
- steel DX51D / Z275 according to EN 10346 with $R_e \geq 220$ N/mm², $R_m \leq 500$ N/mm² and $A_{80} \geq 22\%$
- steel S250GD / Z275 according to EN 10346 with $R_e \geq 250$ N/mm², $R_m \leq 470$ N/mm² and $A_{80} \geq 19\%$
- stainless steel with $R_e \geq 355$ N/mm², $R_m \leq 630$ N/mm² and $A_{80} \geq 22\%$

3.12 General aspects related to the use of the product

The angle brackets are manufactured in accordance with the provisions of this European Technical Assessment using the manufacturing processes as identified in the inspection of the plant by the notified inspection body and laid down in the technical documentation

The nailing pattern used shall be either the maximum or the minimum pattern as defined in Annex A.

The following provisions apply:

- The structural members – the components 1 and 2 shown in the figure on page 15 - to which the brackets are fixed shall be:
  - Restrained against rotation. At a load F_4/F_5, the component 2 is allowed to be restrained against rotation by the Angle brackets.
  - Strength class C14 or better, see section II.2 of this ETA
  - Free from wane under the bracket.
- The actual end bearing capacity of the timber member to be used in conjunction with the bracket is checked by the designer of the structure to ensure it is not less than the bracket capacity and, if necessary, the bracket capacity reduced accordingly.
- The minimum nail's end and edge distances according to EN 1995-1-1:2010 have to be provided for
- The soundproofing interlayer of TITAN SILENT angle brackets shall be arranged between the

horizontal flange and the timber member (component 1 as shown in the figure on page 15).
- There are no specific requirements relating to preparation of the timber members.

The execution of the connection shall be in accordance with the assessment holder’s technical literature.

4 Attestation and verification of constancy of performance (AVCP)

4.1 AVCP system

According to the decision 97/638/EC of the European Commission1, as amended, the system(s) of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) is 2+.

5 Technical details necessary for the implementation of the AVCP system, as foreseen in the applicable EAD

Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited at ETA-Danmark prior to CE marking.

Issued in Copenhagen on 2018-11-06 by

Thomas Bruun
Managing Director, ETA-Danmark

Annex A
Product details definitions

Table A.1 Materials specification

Angel Bracket type	Thickness (mm)	Steel specification	Coating specification
TITAN TTN160	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TTN200	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TTN240	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TTS140	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TTS195	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TTS240	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TCN160	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TCN200	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TCN240	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TCS140	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TCS195	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TCS240	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TTF200	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TCF200	3,0	S235/S275/S355/DX51D/S250GD	FeZn12c / Z 275
TITAN TTV240	4,0	S275/S355 [1)]	FeZn12c / Z 275
TITAN Washer TCW200	12,0	S235/S275/S355	FeZn12c / Z 275
TITAN Washer TCW240	12,0	S235/S275/S355	FeZn12c / Z 275

[1)] or steel DX51D with minimum and maximum mechanical properties of S275 with thickness of 4 mm. An inspection certificate 3.1 according to EN 10204 is required to confirm these values.

Table A.2 Materials specification – Soundproofing Interlayer for TITAN SILENT

Interlayer type	Thickness (mm)
Xylofon or Xylofonplate	6,0
Aladin Stripe Soft	5,0
Aladin Stripe Extrasoft	7,0

Table A.3 Range of sizes

Angle Bracket type	Height (mm) vertical		Height (mm) horizontal		Width (mm)	
TITAN TTN160	119	121	92	94	159	161
TITAN TTN200	119	121	92	94	199	201
TITAN TTN240	119	121	92	94	239	241
TITAN TTS140	129	131	129	131	139	141
TITAN TTS195	129	131	129	131	194	196
TITAN TTS240	129	131	129	131	239	241
TITAN TCN160	119	121	102	104	159	161
TITAN TCN200	119	121	102	104	199	201
TITAN TCN240	119	121	122	124	239	241
TITAN TCS140	129	131	102	104	139	141
TITAN TCS195	129	131	102	104	194	196
TITAN TCS240	129	131	122	124	239	241
TITAN TTF200	70	72	70	72	199	201
TITAN TCF200	70	72	102	104	199	201
TITAN TTV240	119	121	92	93	239	241
TITAN Washer TCW200	-	-	71	73	189	191
TITAN Washer TCW240	-	-	73	75	229	231

Table A.4 Fastener specification

Fastener	**Minimum Length**	**Minimum Threaded Length**	**Fastener type**
Nail 4.0 mm	60 mm	50 mm	Ringed shank nails according to EN 14592 or ETA
Rotho Blaas screw 5.0 mm, type LBS	50 mm	46 mm	Self-tapping screws according to ETA-11/0030
Rotho Blaas screw 8.0 mm, type HBS+	80 mm	52 mm	Self-tapping screws according to ETA-11/0030
Rotho Blaas screw 11.0 mm, type VGS	150 mm	140 mm	Self-tapping screws according to ETA-11/0030
Rotho Blaas screw 11.0 mm, type VGS	200 mm	190 mm	Self-tapping screws according to ETA-11/0030

In the load-carrying-capacities of the nailed or with 5.0 mm screwed connection in Annex B the capacities calculated from the formulas of Eurocode 5 are used assuming a thick steel plate when calculating the lateral fastener load-carrying-capacity. For the connection with 8.0 mm screws a thin steel plate is assumed. The load-carrying-capacities of the angle brackets have been determined based on the use of connector nails ø 4.0 mm in accordance with EN 14592 and self-tapping screws according to ETA-11/0030. The characteristic withdrawal capacity of the nails or screws has to be determined by calculation in accordance with EN 1995-1-1:2010, paragraph 8.3.2 (head pull-through is not relevant):

$$F_{ax,Rk} = f_{ax,k} \cdot d \cdot t_{pen}$$ for the nails 4.0 mm

$$F_{ax,Rk} = \frac{n_{ef} \cdot f_{ax,k} \cdot d \cdot \ell_{ef}}{1,2 \cdot \cos^2\alpha + \sin^2\alpha}\left(\frac{\rho_k}{\rho_a}\right)^{0,8}$$ for the screws

where:

n_{ef} Effective number of fasteners
$f_{ax,k}$ Characteristic value of the withdrawal parameter in N/mm^2
d Nail or screw diameter in mm
t_{pen} Penetration depth of the ring shank in mm
ρ_k Characteristic density of the timber in kg/m^3
ρ_a Characteristic density of the timber in kg/m^3 according to $f_{ax,k}$

Based on tests by Versuchsanstalt für Stahl, Holz und Steine, Karlsruhe Institute of Technology, the characteristic value of the withdrawal resistance for the threaded nails used can be calculated as:
$f_{ax,k} = 50 \cdot 10^{-6} \cdot \rho_k^2$

Based on ETA-11/0030 the characteristic value of the withdrawal resistance for the screws type HBS+ d = 8.0 mm, VGS d = 11.0 mm and LBS d = 5.0 mm is:
$f_{ax,k}$ = 11,7 N/mm² (with ρk =350 kg/m^3)

The shape of the nail or screw directly under the head shall be in the form of a truncated cone with a diameter under the head which fits or exceeds the hole diameter.

Bolts diameter	**Correspondent hole diameter**	**Bolts type**
12.0 or 16.0 mm	Max. 2 mm larger than the bolt diameter	See specification of the manufacturer

Metal Anchors diameter	**Correspondent Hole diameter**	**Anchors type**
12.0 or 16.0 mm	Max. 2 mm larger than the anchor diameter	See specification of the manufacturer

Annex B
Characteristic load-carrying capacities and slip moduli

Table B.1: Force F_1, 1 angle bracket / connection timber to concrete or steel

TITAN Type	timber		steel		Bolts inner row	concrete	$K_{1,ser}$ [kN/mm]
	capacity per fastener in the vertical flange $F_{v,Rk}$ [kN] $F_{1,Rk} = n_{ef} \cdot F_{v,Rk}$ [kN]		$F_{1,Rk}$ [kN]		$k_{t,II}$	ℓ_D [mm]	
TCN200	30 screws Ø5 x 50 [1]		11,8		1,09	7,3	3,0 [3]
	-						
TCN200 + Washer TCW200	Nail Ø4 x 60	Screw Ø5 x 50	Washer S235	Washer S355	1,09	7,3	-
	1,93	2,27	45,7	69,0			
TCN240	36 screws Ø5 x 50 [1]		14,1		1,08	6,5	4,1 [3]
	-						
TCN240 + Washer TCW240	Nail Ø4 x 60	Screw Ø5 x 50	Washer S235	Washer S355	1,08	6,5	-
	1,93	2,27	69,8	105,4			
TCS 240	14 screws Ø8 x 80 [2]		16,2		1,08	6,5	5,5 [3]
	-						
TCS 240 + Washer TCW240	14 screws Ø8 x 80 [2]		75,9		1,08	6,5	11,5 [3]
	-						

[1] Number of fasteners in the vertical flange may be reduced. In this case, the load-carrying capacity for a steel-to-timber connection is $F_{1,Rk} = n_{ef} \cdot 1{,}93$ kN for nails Ø4 x 60 or $F_{1,Rk} = n_{ef} \cdot 2{,}27$ kN for screws Ø 5 x 50. Fasteners must be arranged symmetrically.
[2] Number of screws in the vertical flange may be reduced. In this case, the load-carrying capacity for a steel-to-timber connection is $F_{1,Rk} = n_{ef} \cdot 3{,}77$ kN for screws Ø 8 x 80. Screws must be arranged symmetrically.
[3] Value is only valid when using the full number of fasteners given in column “timber”.

Table B.2: Force F_1, 1 angle bracket with washer / connection timber to timber

TITAN Type	timber			steel	Bolts inner row
	capacity per fastener in the vertical flange $F_{v,Rk}$ [kN] $F_{1,Rk} = n_{ef} \cdot F_{v,Rk}$ [kN]		$F_{1,Rk}$ [kN]	$F_{1,Rk}$ [kN]	$k_{t,II}$
TCN200 + Washer TCW200	Nail Ø4 x 60	Screw Ø5 x 50	13,2	49,8	1,07
	1,93	2,27			
TCN240 + Washer TCW240	Nail Ø4 x 60	Screw Ø5 x 50	17,7	63,1	1,05
	1,93	2,27			
TCS240 + Washer TCW240	Screw Ø8 x 80		17,7	63,1	1,05
	3,77				

Table B.3: Force F_1, 1 angle bracket without washer / connection timber to timber

TITAN Type	**Number of fasteners**		**timber**			
			$F_{1,Rk}$ [kN]			$K_{1,ser}$ [kN/mm]
	n_V	n_H	nails Ø4 x 60	screws Ø5 x 50	screws Ø8 x 80	Screws or nails
TTN240	36	36	7,37	16,2	-	36 screws Ø5 x 50: 11,5
TTS240	14	14	-	-	10,3	14 screws Ø8 x 80: 4,8
TTV240 full	36	30 + 5[1)]	101	101	-	Full nailing: 12,5
TTV240 partial	24	24 + 5[2)]	64,5	64,5	-	Partial nailing: 10,5

[1)] with 5 screws 11 x 200 mm (see Figure B. 22, Annex B)
[2)] with 5 screws 11 x 150 mm (see Figure B. 22, Annex B)

Table B.4: Force $F_{2/3}$, 1 angle bracket / connection timber to timber

TITAN Type	**Number of fasteners**		**Timber**			
			$F_{2/3,Rk}$ [kN]			**$K_{2/3,ser}$ [kN/mm]**
	number n_V	**number n_H**	Nails Ø4 x 60	Screws Ø5 x 50	Screws Ø8 x 80	
TTN160	24	24	19,3	24,0	-	-
TTN200	30	30	28,0	34,7	-	-
TTN240	36	36	37,9	46,7	-	-
TTN240 + Xylofonplate	36	36	24,8	22,8	-	-
TTN240 + Aladin Stripe Soft	36	36	28,9	27,5	-	-
TTN240 + Aladin Stripe Extrasoft	36	36	27,5	25,8	-	-
TTS140	8	8	-	-	10,7	-
TTS195	11	11	-	-	17,1	-
TTS240	14	14	-	-	60,0	5,6
TTS240 + Xylofonplate	14	14	-	-	12,5	-
TTS240 + Aladin Stripe Soft	14	14	-	-	14,7	-
TTS240 + Aladin Stripe Extrasoft	14	14	-	-	13,9	-
TTF200, h=9cm[1)]	30	30	35,5	42,5	-	-
TTF200, h=8cm[1)]	25	25	31,0	37,2	-	-
TTF200, h=7cm[1)]	15	15	20,9	25,1	-	-
TTF200, h=6cm[1)]	10	10	15,1	18,1	-	-
TTF200 + Xylofonplate	30	30	17,2	15,8	-	-
TTF200 + Aladin Stripe Soft	30	30	20,0	19,0	-	-
TTF200 + Aladin Stripe Extrasoft	30	30	19,0	17,9	-	-
TTV240 full	36	30 + 2[2)]	59,7	59,7	-	Full nailing: 6,6
TTV240 partial	24	24 + 2[3)]	51,5	51,5	-	Partial nailing: 4,8

[1)] h = height of purlin (see Figure Figure B. 20, Annex B)
[2)] with 2 screws 11 x 200 mm (see Figure B. 22, Annex B)
[3)] with 2 screws 11 x 150 mm (see Figure B. 22, Annex B)

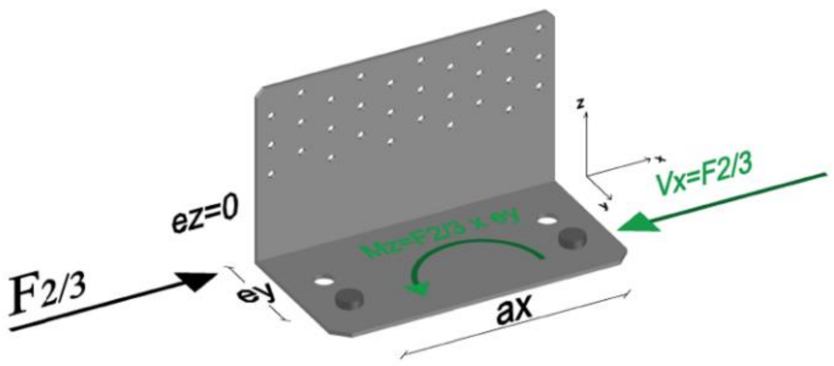
Table B.5: Force $F_{2/3}$, 1 angle bracket with washer / connection timber to timber

TITAN Type	Number of fasteners		Timber $F_{2/3,Rk}$ [kN]			Bolts inner row		a_x
	number n_V	number n_H	Nails Ø4 x 60	Screws Ø5 x 50	Screws Ø8 x 80	$k_{t,\perp}$	e_y	
TCN 200 + TCW 200	30	2	22,1	26,5	-	0,56	38,5	150
TCN 240 + TCW 240	36	2	30,3	36,3	-	0,56	39,5	162
TCS 240 + TCW 240	14	2	-	-	25,0	0,56	39,5	162

Table B.6: Force $F_{2/3}$, 1 angle bracket / connection timber to concrete or steel

TITAN Type	Number of fasteners		Timber $F_{2/3,Rk}$ [kN]			Bolts inner row					$K_{2/3,ser}$ [kN/mm]
	number n_V	number n_H	Nails Ø4 x 60	Screws Ø5 x 50	Screws Ø8 x 80	$k_{t,\perp}$	e_y	$k_{t,\parallel}$	e_z	a_x	Screws
TCN 200 + TCW 200	30	2	56,7	66,4	-	0,56	38,5	0,56	83,5	150	9,6
TCN 240 + TCW 240	36	2	70,5	82,6	-	0,56	39,5	0,52	83,5	162	10
TCS 240 + TCW 240	14	2	-	-	85,9	0,56	39,5	0,48	78,5	162	8,6

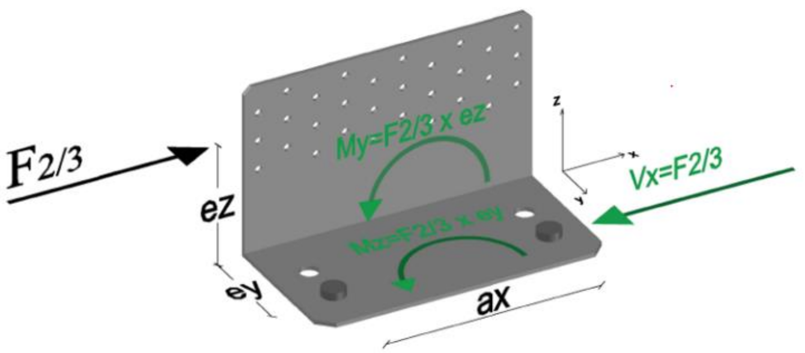
Explanation for table B.5 and B.7



$F_{2/3}$

$M_z = F_{2/3} * e_y$

Explanation for table B.6



$F_{2/3}$

$M_z = F_{2/3} * e_y$

$M_y = F_{2/3} * e_z$

Table B.7: Force $F_{2/3}$, 1 angle bracket / connection timber to concrete or steel

TITAN Type	Number of fasteners		Timber $F_{2/3,Rk}$ [kN]			Bolts inner row		Bolts outer row			$K_{2/3,ser}$ [kN/mm]
	number n_V	number n_H	Nails Ø4 x 60	Screws Ø5 x 50	Screws Ø8 x 80	$k_{t,\perp}$	e_y	$k_{t,\perp}$	e_y	a_x	
TCN160	24	2	15,1	18,1	-	0,61	38,5	0,81	70,0	110	-
TCN200	30	2	22,1	26,5	-	0,56	38,5	0,68	70,0	150	-
TCN200 (Nailing pattern 1)	10	2	6,38	7,48	-	0,56	38,5	0,68	70,0	150	-
TCN200 (Nailing pattern 2)	15	2	9,58	11,2	-	0,56	38,5	0,68	70,0	150	-
TCN200 (Nailing pattern 3)	20	2	13,7	16,0	-	0,56	38,5	0,68	70,0	150	-
TCN200 (Nailing pattern 4)	25	2	17,4	20,4	-	0,56	38,5	0,68	70,0	150	-
TCN240	36	2	30,3	36,3	-	0,56	39,5	0,70	80,5	150	-
TCN240 (Nailing pattern 1)	12	2	8,85	10,4	-	0,56	39,5	0,70	80,5	162	-
TCN240 (Nailing pattern 2)	18	2	13,3	15,6	-	0,56	39,5	0,70	80,5	162	-
TCN240 (Nailing pattern 3)	24	2	18,8	22,1	-	0,56	39,5	0,70	80,5	162	-
TCN240 (Nailing pattern 4)	30	2	24,0	28,2	-	0,56	39,5	0,70	80,5	162	-
TCS140	8	2	-	-	10,7	0,66	38,5	0,92	70,00	90	-
TCS195	11	2	-	-	17,1	0,56	38,5	0,68	70,00	150	-
TCS240	14	2	-	-	70,3	0,56	39,5	0,70	80,5	162	8,2
TCF200, h=9cm[1)]	30	2	35,5	42,5	-	0,56	38,5	0,68	70,00	150	-
TCF200, h=8cm[1)]	25	2	31,0	37,2	-	0,56	38,5	0,68	70,00	150	-
TCF200, h=7cm[1)]	15	2	20,9	25,1	-	0,56	38,5	0,68	70,00	150	-
TCF200, h=6cm[1)]	10	2	15,1	18,1	-	0,56	38,5	0,68	70,00	150	-

[1)] h = height of purlin (see Figure Figure B. 20, Annex B)

Table B.8: Force $F_{4/5}$, 2 angle brackets

TITAN Type	Number of fasteners		$F_{4/5,Rk}$ [kN]		Bolts inner row	
	number n_V	number n_H	timber	steel	$k_{t,\perp}$	$k_{t,\parallel}$
TTN240	72	72	26,7	31,6	-	-
TTS240	28	28	25,2	23,4	-	-
TCN200	60	4	25,6	14,9	0,41	0,09
TCN200 (nailing pattern 2)	30	4	22,4	20,9	0,46	0,06
TCN240	72	4	27,8	24,7	0,43	0,06
TCN240 (nailing pattern 2)	36	4	25,2	30,6	0,48	0,04
TCS240	72	4	27,4	18,8	0,39	0,08
TTF200	60	60	21,0	14,2	-	-
TCF200	60	4	23,8	12,3	0,31	0,10

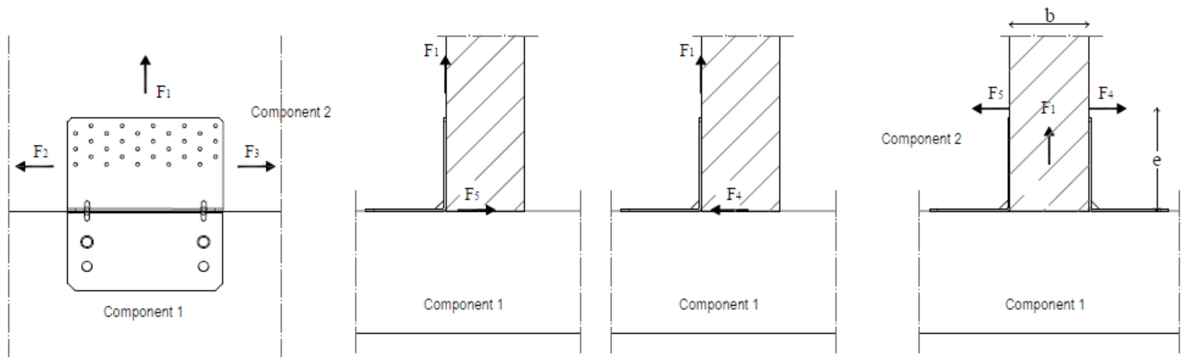
Table B.9: Force F_4, 1 angle bracket

TITAN Type	**Number of fasteners**		**$F_{4,Rk}$ [kN]**		**Bolts inner row**	
	number n_V	**number n_H**	timber	steel	$k_{t,\perp}$	$k_{t,\parallel}$
TTN240	36	36	23,8	31,1	-	-
TTS240	14	14	20,7	20,9	-	-
TCN200	30	2	20,9	22,4	0,50	-
TCN200 (nailing pattern 2)	15	2	20,7	24,3	0,50	-
TCN240	36	2	24,1	26,9	0,50	-
TCN240 (nailing pattern 2)	18	2	23,9	29,1	0,50	-
TCS240	36	2	21,1	18,1	0,50	-
TTF200	30	30	14,1	10,4	-	-
TCF200	30	2	14,6	9,48	0,50	-

Table B.10: Force F_5, 1 angle bracket

TITAN Type	**Number of fasteners**		**$F_{5,Rk}$ [kN]**		**Bolts inner row**	
	number n_V	**number n_H**	timber	steel	$k_{t,\perp}$	$k_{t,\parallel}$
TTN240	36	36	7,26	3,41	-	-
TTS240	14	14	16,8	4,24	-	-
TCN200	30	2	6,64	2,74	0,50	0,47
TCN200 (nailing pattern 2)	15	2	3,60	1,58	0,50	0,83
TCN240	36	2	8,02	3,28	0,50	0,48
TCN240 (nailing pattern 2)	18	2	4,33	1,89	0,50	0,83
TCS240	36	2	17,1	4,30	0,50	0,36
TTF200	30	30	10,8	4,69	-	-
TCF200	30	2	10,7	4,77	0,50	0,27

Definitions of forces, their directions and eccentricity
Forces – Example:



Fastener specification
Nailing patterns are given in figures B.20, B.21 and B.22. Unless otherwise stated the load-carrying capacities are applicable for connections with nails ø 4.0 mm as well as for LBS screws ø 5.0 mm. In Connections with bolts or metal anchors, there are always two bolts/metal anchors per angle bracket. Unless otherwise stated, their position is in the first row from the bend line (inward).

Double angle brackets per connection
The angle brackets must be placed at each side opposite to each other, symmetrically to the component axis.
Acting forces

F_1	Lifting force acting along the central axis of the joint.
F_2 and F_3	Lateral force acting in the joint between the component 2 and component 1 in the component 2 direction
F_4 and F_5	Lateral force acting in the component 1 direction along the central axis of the joint. The load may be applied with an eccentricity e, then a design for combined loading is required.

Single angle bracket per connection
Acting forces

F_1	Lifting force acting in the central axis of the angle bracket. The component 2 shall be prevented from rotation. If the component 2 is prevented from rotation the load-carrying capacity will be half of a connection with double angle brackets.
F_2 and F_3	Lateral force acting in the joint between the component 2 and the component 1 in the component 2 direction. The component 2 shall be prevented from rotation. If the component 2 is prevented from rotation the load-carrying capacity will be half of a connection with double angle brackets.
F_4 and F_5	Lateral force acting in the component 1 direction along the central axis of the joint. The components must be prevented from rotation. F_4 causes compression between the angle bracket and component 2; F_5 causes tension between the angle bracket and component 2.

Wane
Wane is not allowed, the timber has to be sharp-edged in the area of the angle brackets.

Timber splitting
For the lifting force F_1 it must be checked in accordance with Eurocode 5 or a similar national Timber Code that splitting will not occur.

Combined forces
If the forces F_1 and F_2/F_3 or F_4/F_5 act at the same time, the following inequality shall be fulfilled:

$$\left(\frac{F_{1,Ed}}{F_{1,Rd}}\right)+\left(\frac{F_{2,Ed}}{F_{2,Rd}}\right)+\left(\frac{F_{3,Ed}}{F_{3,Rd}}\right)+\left(\frac{F_{4,Ed}}{F_{4,Rd}}\right)+\left(\frac{F_{5,Ed}}{F_{5,Rd}}\right)\leq 1$$

The forces F_2 and F_3 or F_4 and F_5 are forces with opposite direction. Therefore only one force F_2 or F_3, and F_4 or F_5, respectively, is able to act simultaneously with F_1, while the other shall be set to zero.

If the load $F_{4/5,Ed}$ is applied with an eccentricity e, a design for combined loading **for connections with double angle brackets** is required. Here, an additional force ΔF_1 has to be added to the existing force F_1.

$$\Delta F_{1,Ed} = F_{4/5,Ed} \cdot \frac{e}{b}$$

b is the width of component 2.

Bolt design
The load $F_{B,Ed}$ for the design of the maximally loaded bolt or metal anchor is calculated as:

$$F_{B,t,Ed} = k_{t,\parallel} \cdot F_{Ed}$$

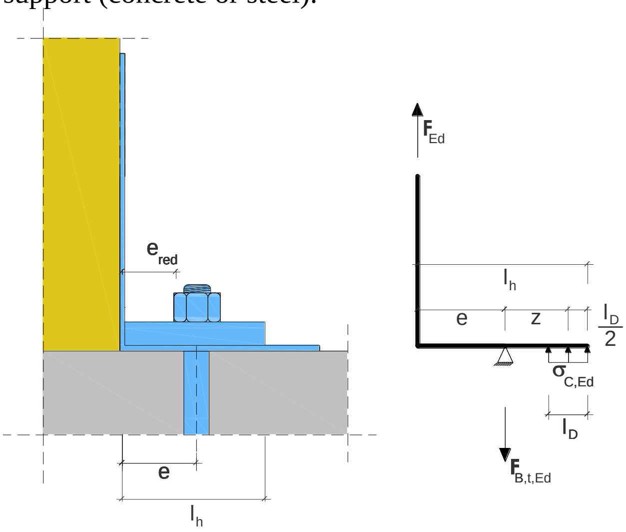
$$F_{B,v,Ed} = k_{t,\perp} \cdot F_{Ed}$$

where:

$F_{B,t,Ed}$	Resulting tensile load on the maximally loaded bolt in the group in N
$F_{B,v,Ed}$	Resulting shear load on the maximally loaded bolt in the group in N
$k_{t,\parallel}$	Coefficient taking into account the resulting axial force
$k_{t,\perp}$	Coefficient taking into account the moment arm or hole tolerance, respectively
F_{Ed}	Load on vertical flap of the angle bracket or pair of angle brackets in N

Load combinations have to be considered.

Compressive stress on the support (concrete or steel):



$$\sigma_{C,Ed} = \frac{F_{1,Ed} \cdot \left(2 \cdot k_{t,\parallel} - 1\right)}{b \cdot \ell_D}$$

where:

$F_{1,Ed}$	Tensile load F_1 on vertical flap of the angle bracket in N
b	Width of the washer in mm
$\sigma_{C,Ed}$	compressive stress on the support (concrete or steel) in N/mm^2

Rotho Blaas Angle Brackets

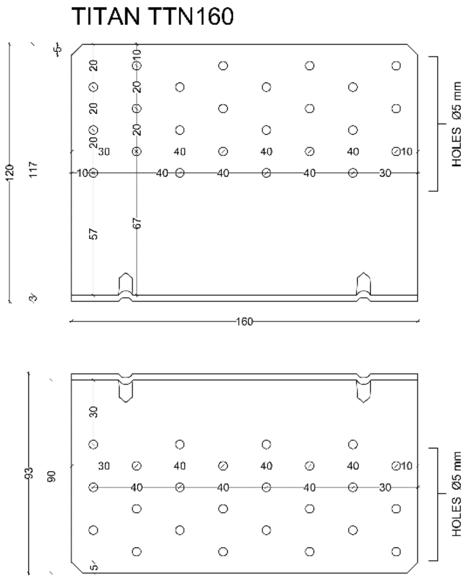


Figure B. 1
Dimensions of Angle Bracket TITAN TTN160

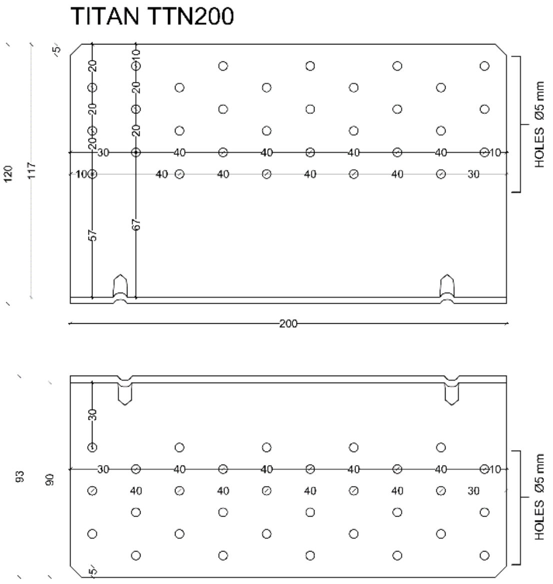


Figure B. 2
Dimensions of Angle Bracket TITAN TTN200

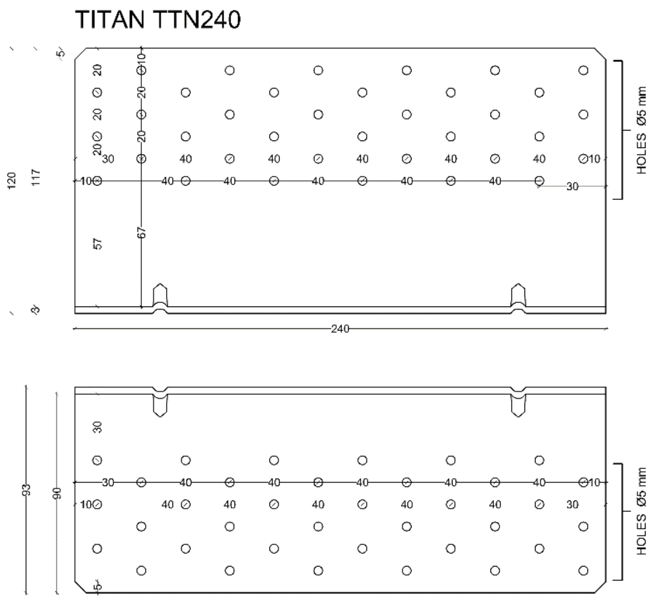


Figure B. 3
Dimensions of Angle Bracket TITAN TTN240

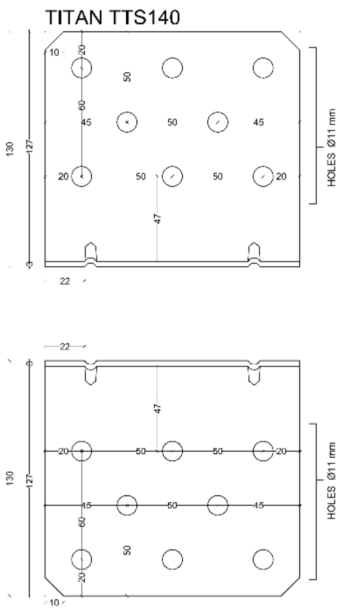


Figure B. 4
Dimensions of Angle Bracket TITAN TTS140

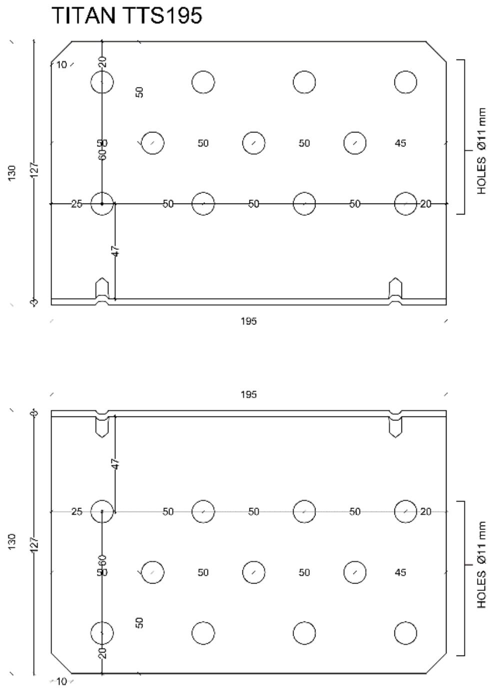


Figure B. 5
Dimensions of Angle Bracket TITAN TTS195

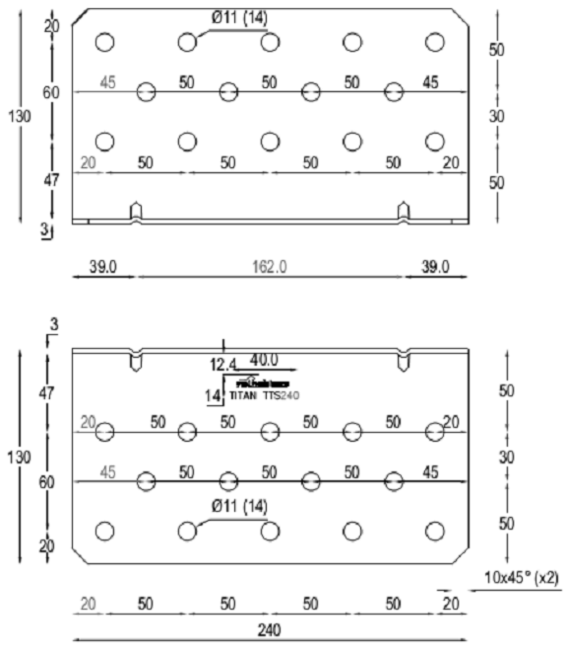


Figure B. 6
Dimensions of Angle Bracket TITAN TTS240

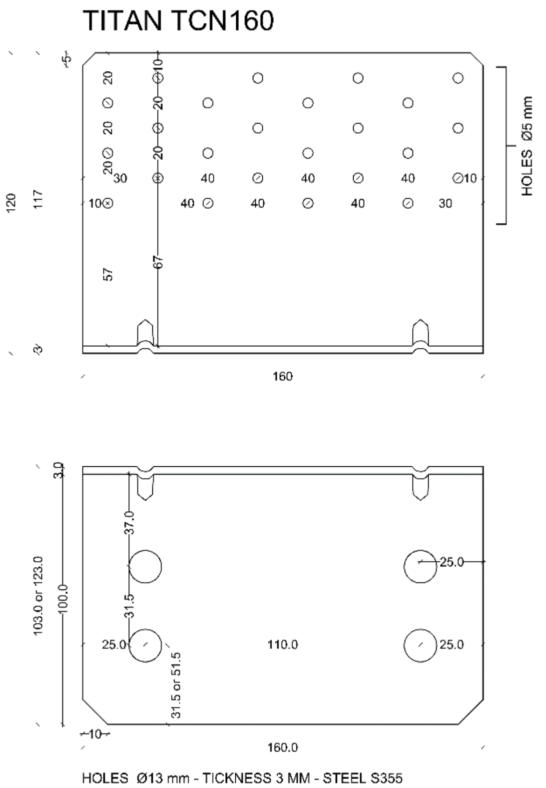


Figure B. 7
Dimensions of Angle Bracket TITAN TCN160

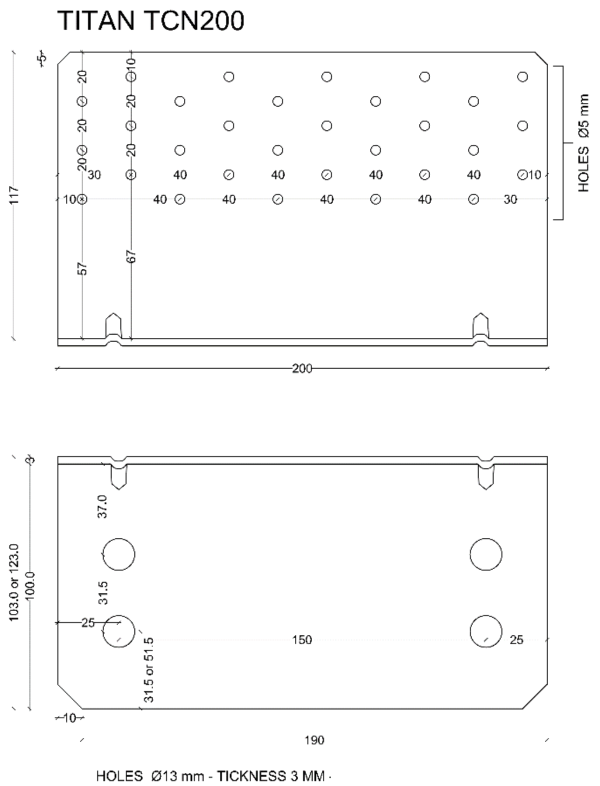


Figure B. 8
Dimensions of Angle Bracket TITAN TCN200

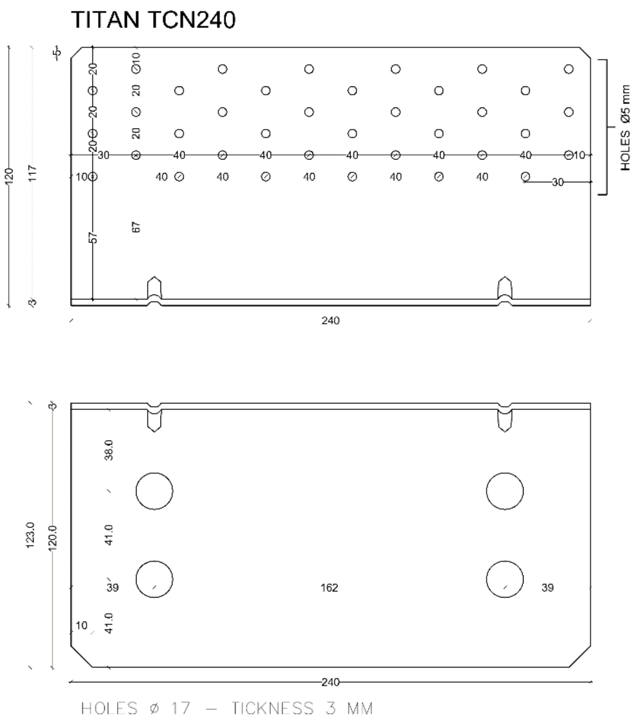


Figure B. 9
Dimensions of Angle Bracket TITAN TCN240

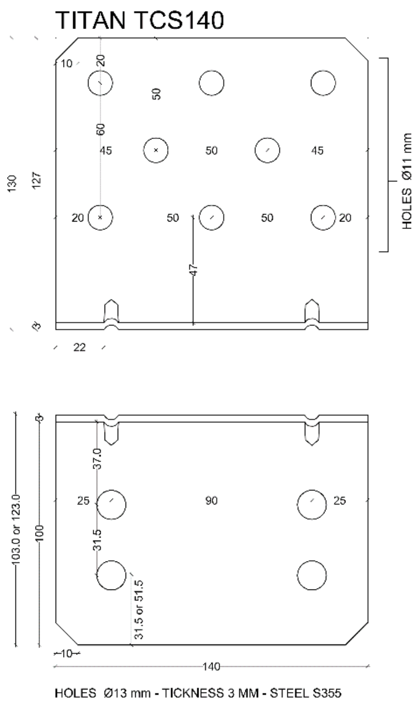


Figure B. 10
Dimensions of Angle Bracket TITAN TCS140

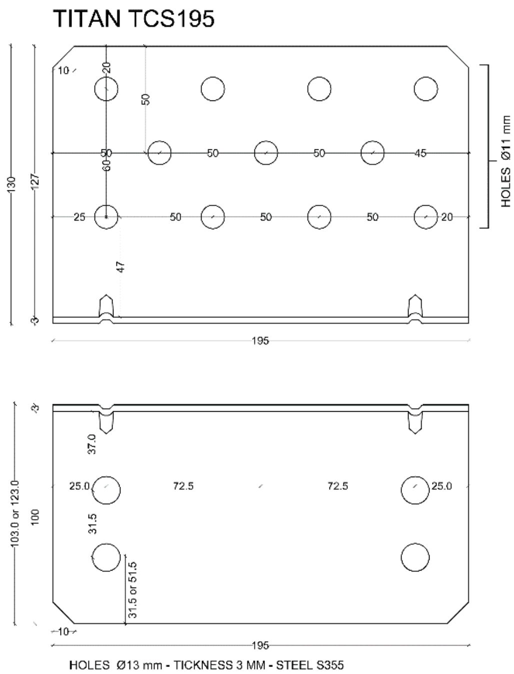


Figure B. 11
Dimensions of Angle Bracket TITAN TCS195

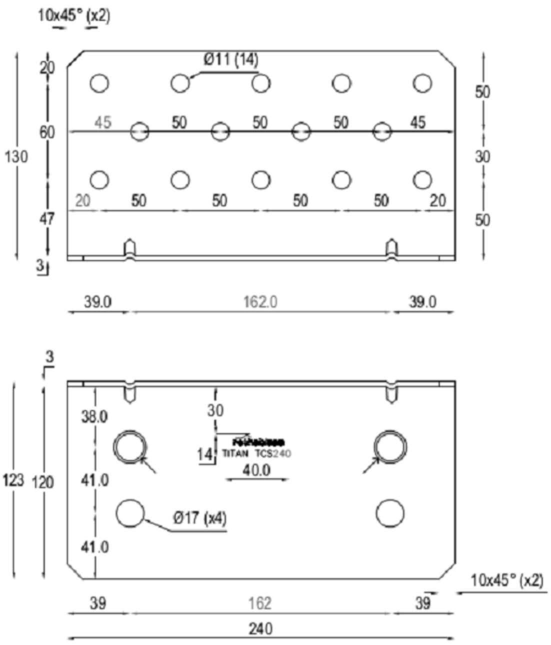


Figure B. 12
Dimensions of Angle Bracket TITAN TCS240

TITAN TTF200

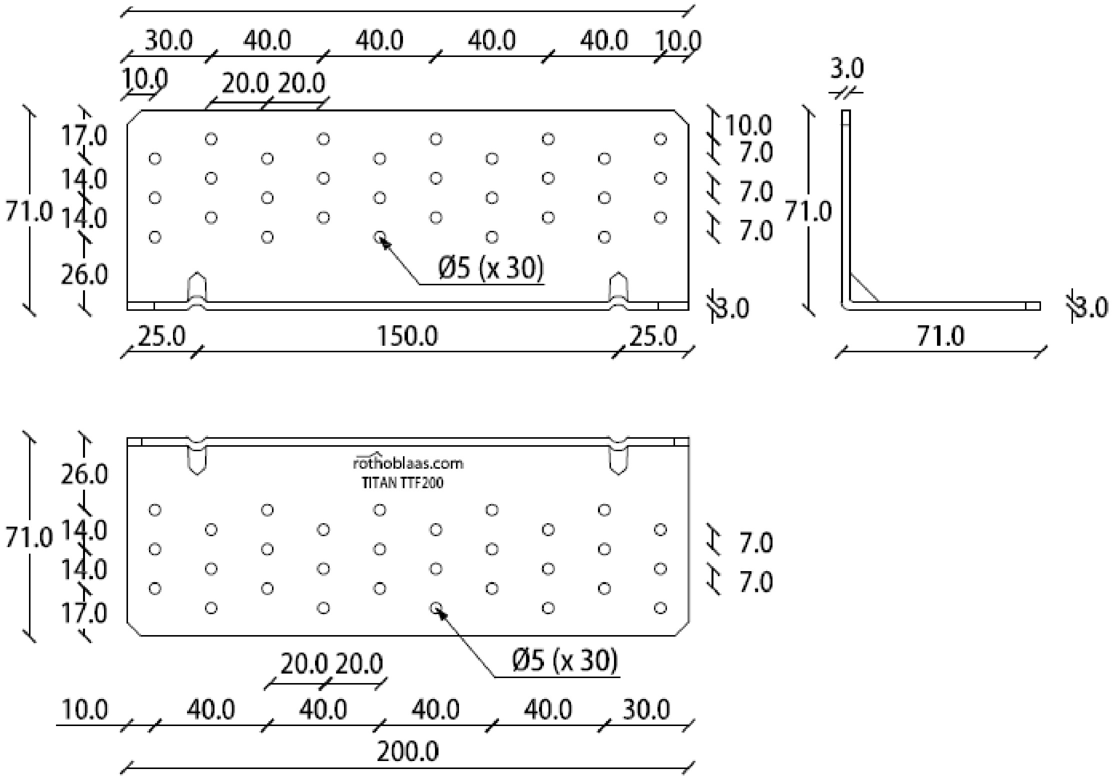


Figure B. 13 Dimensions of Angle Bracket TITAN TTF200

TITAN TCF200

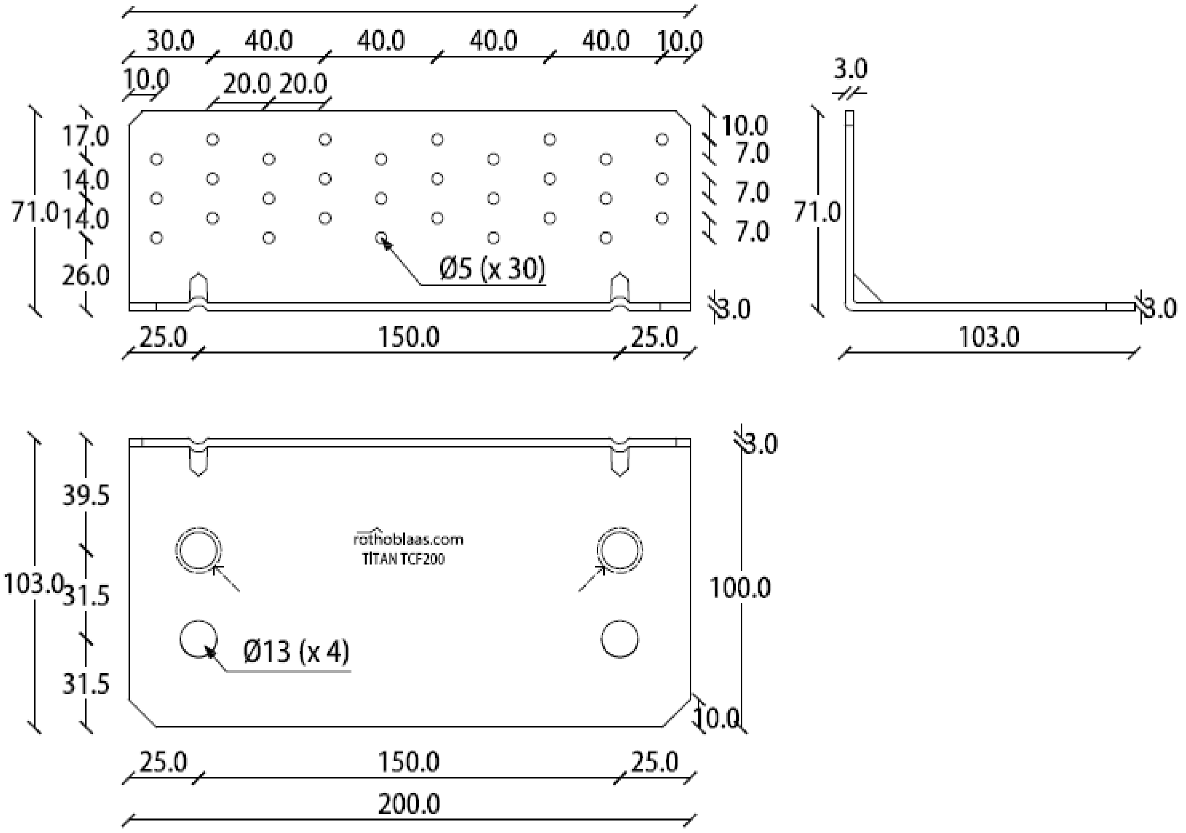


Figure B. 14 Dimensions of Angle Bracket TITAN TCF200

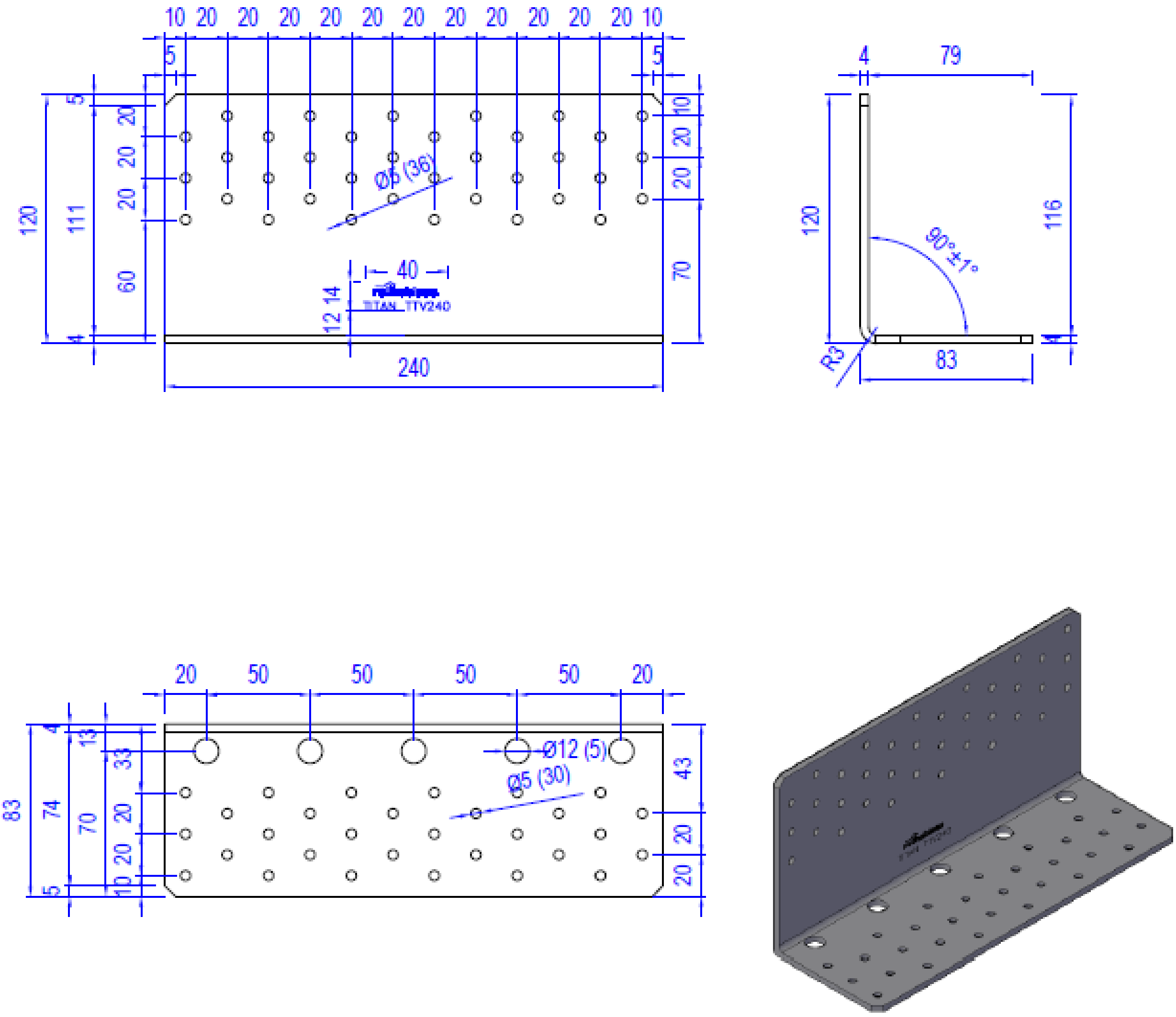


Figure B. 15 Dimensions of Angle Bracket TITAN TTV240

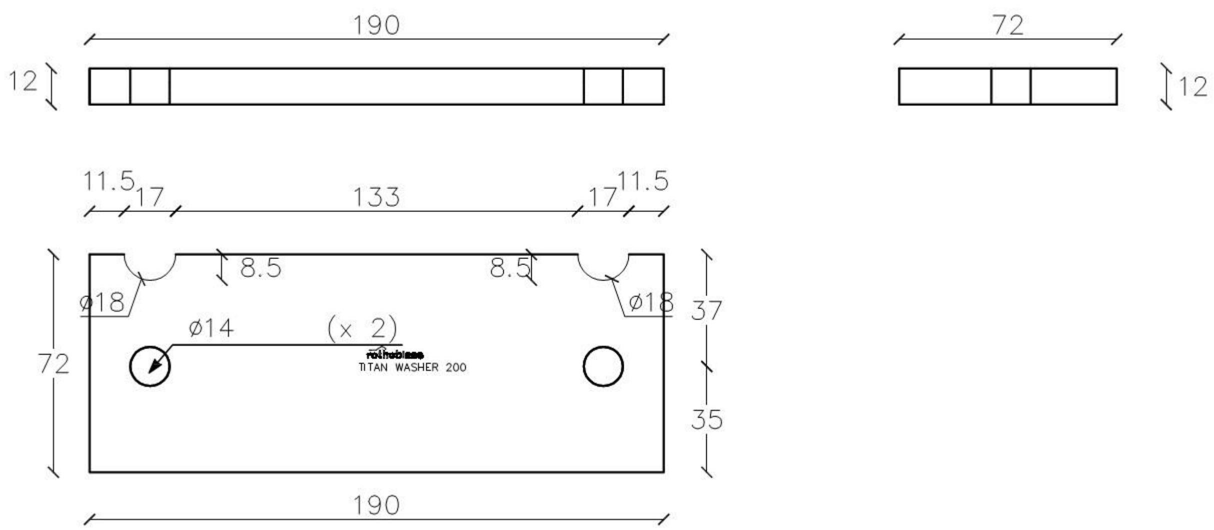


Figure B. 16 Dimensions of TITAN Washer TCW200

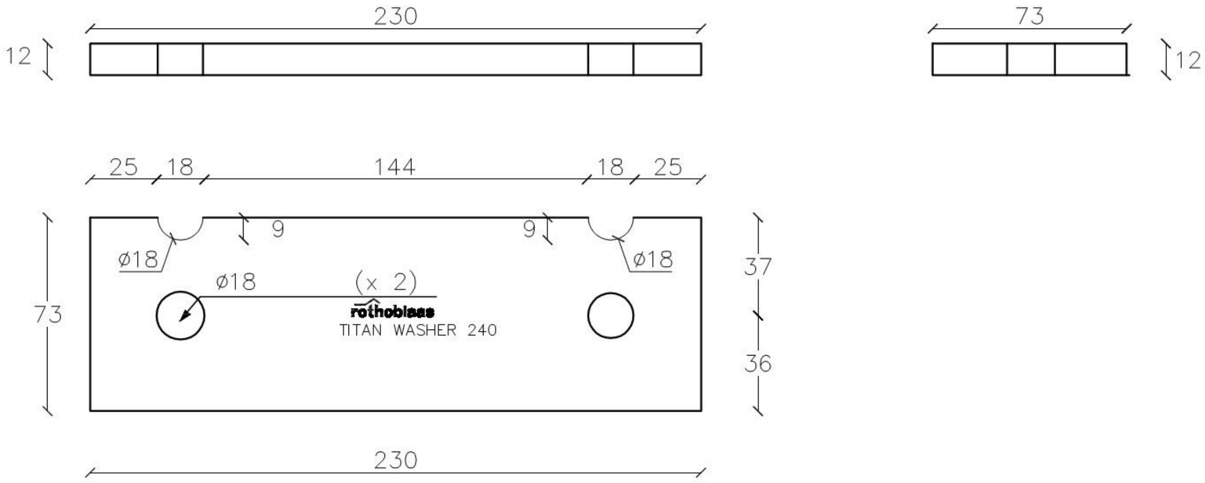
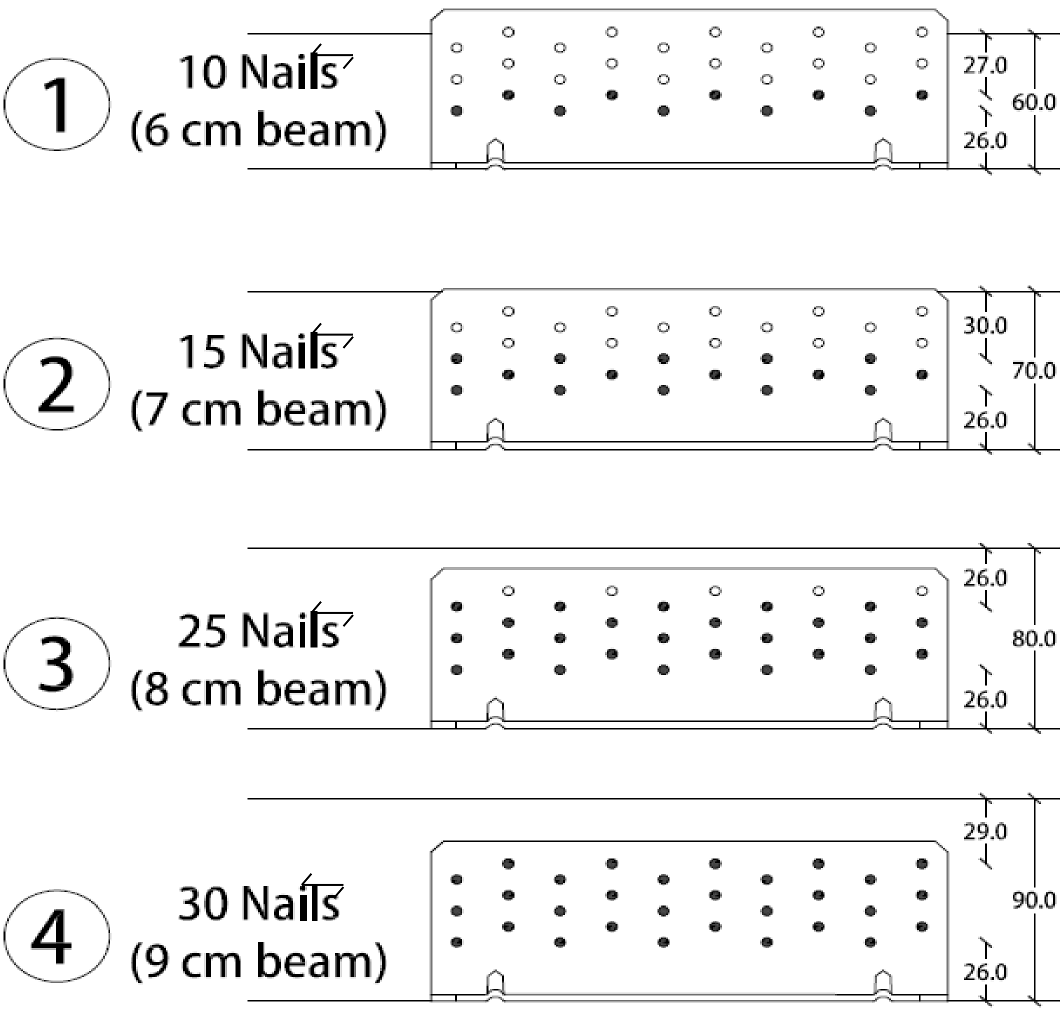


Figure B. 17 Dimensions of TITAN Washer TCW240

Figure B. 18 Typical installation on concrete

Figure B. 19 Typical installation TITAN Silent



In timber to timber connection horizontal flange can be fully nailed or optimized in function of vertical nailing

Figure B. 20 Nailing patterns for Angle Bracket TITAN TTF200 and TCF200
(for TTF200: symmetrical hole-pattern for horizontal and vertical flange)

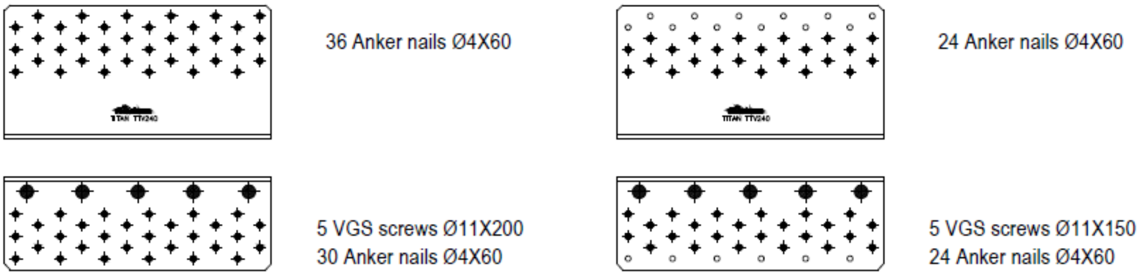
Partial Nailing pattern for TCN200 and TCN240 - F2/3

Figure B. 21 Nailing patterns for Angle Bracket TITAN TCN200 and TCN240

Nailing pattern for TTV240 - F1

FULL NAILING PATTERN
36+30 NAILS and 5 VGS SCREWS

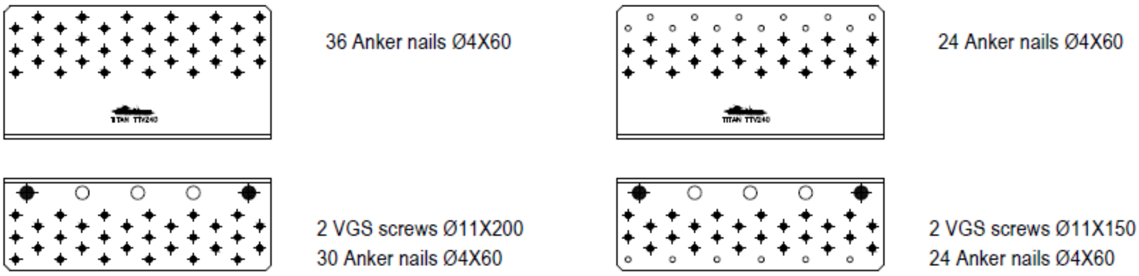
PARTIAL NAILING PATTERN
24+24 NAILS and 5 VGS SCREWS



Nailing pattern for TTV240 - F2/3

FULL NAILING PATTERN
36+30 NAILS and 2 VGS SCREWS

PARTIAL NAILING PATTERN
24+24 NAILS and 2 VGS SCREWS



Insertion angle VGS screw for TTV240

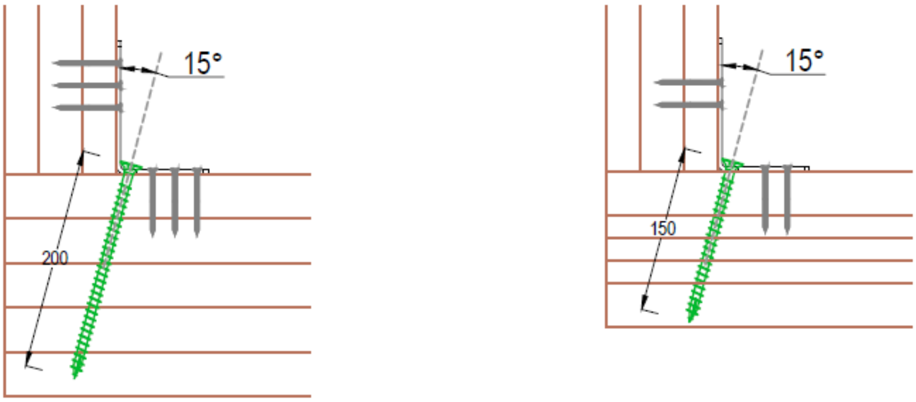


Figure B. 22 Nailing patterns for Angle Bracket TITAN TTV240

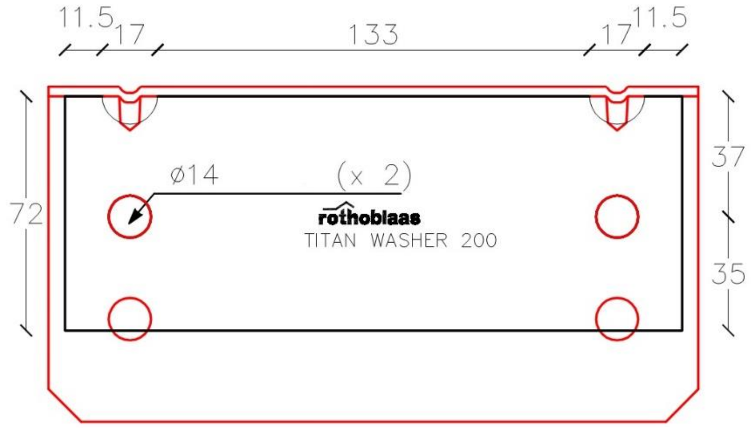


Figure B. 23 Typical installation for Angle Bracket TITAN TCN200 with Washer

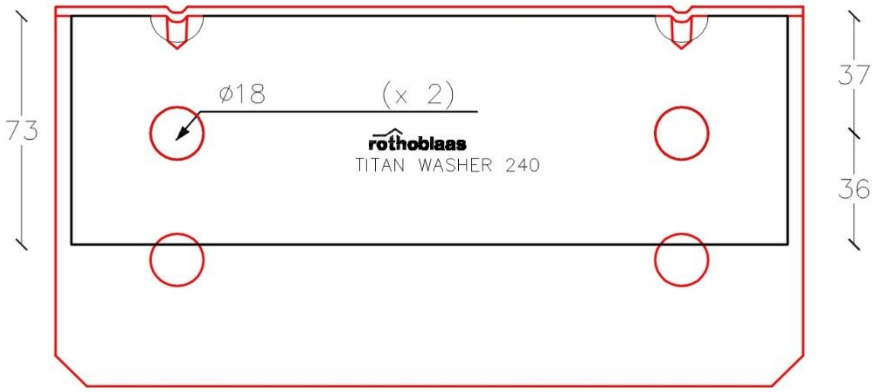


Figure B. 24 Typical installation for Angle Bracket TITAN TCN240 with Washer